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# A GIS- Independent method for Prediction of Stream-Order-Law Ratios Prediction in geomorphologic parameters of catchment Catchment with GIS to for Runoff estimate Estimation runoff using using GIUH model

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## **Abstract:**

Estimation of ~~flow~~ direct runoff in ungauged catchments has great importance in the d  
hydraulic structures. The geomorphologic instantaneous unit hydrograph (GIUH) technic  
geomorphologic parameters to estimate catchment runoff. In this research, regression e  
were developed based on ~~morphometric geometrical~~ characteristics of ~~nine~~ some catchme  
as area, length and slope of the main river to estimate stream-order-law rat  
geomorphologic ~~data characteristics~~ of other catchments with no need for GIS and  
elevation model. These equations were verified ~~used for verification of stream order la~~  
~~as well as geomorphologic parameters corresponding to in~~ the Gagas, Heng-Chi and  
catchments. In this study, the effect of stream-order-law ratios on the rate of runoff in  
catchment was examined, and the sensitivity of runoff rate to each ratio was analyz  
GIUH model was assessed in two cases of GIS-supported and ~~GIS unsupported~~  
method. The mean errors of the regression equations in estimation of ratios  $R_B$ ,  $R_L$ ,  $R_A$   
 $R_{SO}$  in three study catchments were 4.7%, 23.5%, 7.1%, 41.3%, and 22.9%, respecti

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29 direct runoff hydrograph for the Heng-Chi and the Kasilian catchments were computed b  
30 model and compared with observed direct runoff. According to the results, the errors  
31 discharge for four rainfall-runoff events in ~~GIS-unsupported~~proposed method case v  
32 average, 10% more than the error in the case of GIS-supported GIUH. The results of G  
33 the two cases are very close to each other. The mean coefficient of efficiency of the mc  
34 computed as 0.87.

35 **Key words:** GIUH, GIS, Stream-order-law ratios, Geomorphologic parameters, Runoff

## 36 1. Introduction

37 Estimation of design flood in catchments is a vital issue in design of flood control st  
38 Most catchments of the world are ungauged and the statistical methods which strongly  
39 rainfall-runoff data are not efficient, hence the rainfall-runoff models are employed to  
40 runoff. GIUH is a rainfall-runoff model for estimating runoff in ungauged catchmen  
41 ~~their~~based on geomorphologic parameters (GP) of catchment.

42 ~~Studies on streams orderings of catchments were first introduced by Horton (1932, 1945~~  
43 ~~modifications were made on Horton's method by Strahler (1952, 1957, 1964) leading t~~  
44 ~~method of ordering.~~

45 The idea of GIUH was introduced by Rodriguez-Iturbe and Valdes (1979). They sugg  
46 instantaneous unit hydrograph (IUH) model in which time to peak and peak flow  
47 catchment were functions of geomorphologic features. ~~The geomorphologic parameter~~  
48 ~~catchments are calculated by GIS software such as ArcGis and hydrologic extensions~~  
49 ~~ArcHydro. For this purpose, DEM of the catchment is necessary. First, stream netw~~  
50 ~~delineated and, GP such as the number of streams, lengths, slopes, and drainage areas~~  
51 ~~order of streams is carried out based on stream orderings (Horton Strahler method).~~

52 GIUH model was extended and used by other scientists in different catchments (e.g. Guj  
53 1980; Rodriguez-Iturbe et al. 1982; Lee and Yen 1997 and Kumar and Kumar 2008, S.  
54 and Norouzpoor 2014).

55 An alternative approach was provided by Lee and Yen (1997). The travel times for c  
56 orders of overland areas and channels were derived using the kinematic-wave theory a

57 substituted into the GIUH model to develop a kinematic wave-based GIUH model for w  
58 runoff simulation.

59 Lee and Chang (2005) offered a GIUH model to estimate surface and subsurface  
60 catchments. In their research, special importance was given to separation of surface fl  
61 subsurface flow in catchments. Sabzevari et al. (2013) modified the model presented by  
62 Chang (2005) for estimation of surface and subsurface flow of Kasilian catchment. ~~Th~~  
63 ~~also given a saturation model for separation of saturated and unsaturated zones of c~~  
64 ~~regions.~~

65 ~~Sabzevari and Norouzpoor (2014) suggested a GIUH model which is capable of taki~~  
66 ~~shape and profile curvature in complex hillslopes in computation of surface and subsurfa~~  
67 ~~time. Also, the effect of geometry of complex hillslopes on the runoff in sub-catchment~~  
68 ~~of Walnut Gulch was investigated.~~

69 ~~Kumar et al. (2004, 2007) rendered the runoff estimation of ungauged catchments by i~~  
70 ~~the GIUH-based Nash and Clark models. They used stream ratios to estimate Nash and~~  
71 ~~parameters. Kumar and Kumar (2008) focused on estimation of runoff in Ramganga cat~~  
72 ~~India, applying GIUH based on kinematic wave theory. The model was used in the case~~  
73 ~~the inputs were geomorphologic parameters and stream order law ratios. Travel tim~~  
74 ~~streams and overland regions in the two above cases were given as analytic equations t~~  
75 ~~Horton Strahler stream ordering system.~~

76 ~~Choi et al. (2011) used a concept of geomorphologic dispersion to estimate Nash~~  
77 ~~parameters from spatial heterogeneity of flow path within a catchment.~~

78 The geomorphologic parameters of the catchments are calculated by GIS softwares  
79 ArcGis and hydrologic extensions such as ArcHydro. For this purpose, digital elevatio  
80 (DEM) of the catchment is necessary. Stream networks are delineated and, GP sucl  
81 number of streams, lengths, slopes, and drainage areas in each order of streams are c  
82 based on stream orderings.

83 The stream-order-law ratios (SOLR) are calculated based on geomorphologic informa  
84 vice versa. The equations governing the GIUH model can be developed based on SOL  
85 (Kumar & Kumar, 2008).

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	14

Based on GP of catchment, stream order law ratios such as bifurcation ratio ( $R_b$ ), stream ratio ( $R_L$ ), stream area ratio ( $R_A$ ), and stream slope ( $R_s$ ) ratio could be computed. According to the GIUH offered by Yen and Lee (1997), the travel times of overland region stream could be worked out regarding stream-order-law ratios prior to IUH estimation. Studies on streams orderings of catchments were first introduced by Horton (1932, 1945) modifications were made on Horton's method by Strahler (1952, 1957, 1964) leading to method of ordering. Shreve (1966) concluded that the Strahler stream numbers generally gave a better fit for stream networks than did the Horton stream numbers. Horton-Strahler's laws were extensively used in geomorphological applications to classify systems [e.g., Raff et al., 2003; Reis, 2006], to establish relations with the fractal nature of network as detailed by Rodríguez-Iturbe and Rinaldo [1997] [e.g., Beer and Borgas, 1993; La and Roth, 1994; Rodríguez-Iturbe et al., 1994], and to characterize scale properties [Claps et al., 2000; Peckham and Gupta, 1999; Veitzer and Gupta, 2000; Dodds and Rothman, 1999, 2001].

Based on GP of catchment, stream order law ratios such as bifurcation ratio ( $R_b$ ), stream ratio ( $R_L$ ), stream area ratio ( $R_A$ ), and stream slope ( $R_s$ ) ratio could be computed. According to the GIUH offered by Yen and Lee (1997), the travel times of overland region and stream could be worked out regarding stream-order-law ratios prior to IUH estimation.

Due to the lack of topographic map and DEM for most of the catchments, application-based GIUH models is practically useless. One goal of this research is to provide a technique which one could compute geomorphologic parameters without the need for GIS. Calculating GP by means of GIS is costly and takes a long time. For example, extensions such as ArcView though capable of calculating the number, length, and slope of streams at any order, provide information about overland surface slopes or drainage area at any order which ought to be calculated manually by GIS specialists which is time consuming. For this purpose, GP of catchments of various sizes with diverse stream networks were collected. The values of stream order law ratios and the actual GP of the catchments obtained from GIS were derived.

Using GIS tool is one of the best ways of calculating the geomorphologic parameters (Sarang et al., 2003; Obi Reddy et al., 2004; Valeriano et al., 2006; Ozdemir and Bird, 2009).

116 The high resolution of DEM results in a more accurate prediction of GP and runoff using  
 117 models. Although, today the whole world's DEM is prepared by the Shuttle Radar Topog  
 118 Mission, but some of the hydrologists due to their little familiarity with GIS, do not like u  
 119 GIUH methods because extracting the GP requires a lot of time and they apply simpler n  
 120 of rainfall-runoff. In this research, based on geomorphological information of a number o  
 121 catchment a set of equations is provided whose SOLR and GP geomorphologic paramete  
 122 be calculated based on parameters such as catchment's area and main river length. These  
 123 used as input parameters of GIUH model and is used for predicting the surface and subsu  
 124 runoff of the catchment.

125 ~~To study the relation between data, linear and nonlinear regressions were used using tl~~  
 126 ~~software. In general, length and slope of the main stream and area of the catchment are~~  
 127 ~~the geometric parameters that are easily computable for every catchment. It is also imp~~  
 128 ~~present empirical equations which could predict all stream order law ratios based~~  
 129 ~~geometrical catchment information.~~

130 The important aims of this research are:

131 (1) to present equations which can predict, ~~without the use of GIS and DEM of the cat~~  
 132 the stream-order-law ratios of catchment on the basis of length, slope of the main stre  
 133 area of catchment (~~geometrical features~~).

134 ~~(2) to analyze sensitivity of stream ratios and its effect on direct runoff hydrograph (DRH~~

135 ~~(32) Apply of predicted stream-order-law ratios~~ to estimate direct runoff of ungauged cat  
 136 by means of GIUH ~~without the use of GIS method.~~

## 137 **2. GIUH model**

138 Surface runoff of the overland regions moves, through stream networks, to the c  
 139 catchment. If a catchment is ordered via Strahler ordering scheme, the water travel pat

the overland regions to the outlet are specified. Each flow path is comprised of differer  
the first of which is the overland region and the others are the streams. The probability  
motion in a certain path  $w: x_{o_i} \rightarrow x_i \rightarrow x_j \rightarrow \dots \rightarrow x_{\Omega}$  is expressed as:

$$P(w) = P_{OA_i} P_{x_{oi}, x_i} P_{x_i, x_j} \dots P_{x_k, x_{\Omega}}$$

where  $P_{OA_i}$  is the initial state probability of rain drop moving from  $i$ th order overland r  
the  $i$ th order stream, which can be approximated as the ratio of  $i$ th order overland area to  
catchment area;  $P_{x_{oi}, x_i}$  which is the probability of raindrop moving from  $i$ th order overlan  
( $x_{o_i}$ ) to  $i$ th order stream equals one; and  $P_{x_i, x_j}$  is the transitional probability of rain drop  
from  $i$ th order stream ( $x_i$ ) to  $j$ th order channel ( $x_j$ ).

The number of streams at each order and how they are connected to each other spe  
probabilities in Eq. (1).

The value of IUH of a watershed comprising different runoff paths is given by  
(Rodriguez-Iturbe and Valdes 1979).

$$u(t) = \sum_{w \in W} [f_{x_{o_i}}(t) * f_{x_i}(t) * f_{x_j}(t) * \dots * f_{x_{\Omega}}(t)]_w \times P(w)$$

where  $f_{x_k}(t)$  denotes the travel time probability density function (PDF) in state  $x_k$  with  
travel time value ( $T_{x_k}$ ) and the function  $f$  is indeed the IUH of any state  $x_k$  calculatec  
formula  $f(t) = (1/T_{x_k}) \exp(-t/T_{x_k})$ . The PDF is a function of the travel time of each sta  
overland regions and streams. Asterisk (\*) denotes a convolution integral.  $w \in W$ , 1  
 $W = \langle x_{o_i}, x_i, x_j, \dots, x_{\Omega} \rangle$ ,  $i = 1, 2, 3, \dots, \Omega$  and  $t$  is the time.

To solve Eq. (2), one could resort to the Laplace transformations. In the process o  
derivation, computation of travel time is the most intricate part of the work because i  
depends on GP of the catchment.

The ordinates of DRH for the catchment were estimated by convoluting the effective  
hyetograph with the derived IUH.

164 The equation for estimation of DRH is:

$$165 \quad Q(t) = \int_0^t u(t-\tau) I_e(\tau) d\tau \quad (6)$$

166 where  $I_e$  is the excess rainfall and  $u(t)$  is the catchment IUH.

## 167 2.1. Travel time of overland planes and streams

168 According to the kinematic wave theory, the travel time of an overland plane depend  
169 length, slope, Manning coefficient, and excess rainfall intensity. Eq. (4) which is due to  
170 Lee (1997) gives the travel time of the  $i$ th overland plane.

$$T_{X_{oi}} = \left( \frac{n_0 A P_{OA_i} \sum_{i=1}^{\Omega} R_L^{i-\Omega}}{2a^{1/2} S_{c\Omega}^{b/2} L q_L^{m-1} R_B^{\Omega-i} R_L^{i-\Omega} R_S^{b(i-\Omega)/2}} \right)^{1/m}$$

172 where  $R_B$ ,  $R_L$ ,  $R_A$ , and  $R_S$  are bifurcation ratio, stream-length ratio, stream-area ratio, and  
173 slope ratio, respectively;  $A$  is the area of the catchment;  $a$  and  $b$  are 5.463 and  
174 respectively;  $q_L$  is the excess rainfall intensity;  $n_0$  is the Manning's roughness coefficient  
175 overland flow;  $S_{c\Omega}$  is slope of the highest order stream; the constant  $m$  can be recognize  
176 from Manning's equation and  $L$  is sum of mean length of the streams of different orders.  
177 The travel time of the  $i$ th-order channel in each path is obtained, based on its GP, through  
178 (Yen and Lee 1997):

$$T_{X_i} = \frac{B_{\Omega} L R_L^{i-\Omega} R_B^{\Omega-i} \sum_{i=1}^i R_L^{i-\Omega}}{q_L A P_{OA_i} (\sum_{i=1}^{\Omega} R_L^{i-\Omega})^2} \left[ \left( h_{co_i}^m + \frac{q_L A P_{OA_i} n_c \sum_{i=1}^{\Omega} R_L^{i-\Omega}}{B_{\Omega} S_{c\Omega}^{1/2} R_S^{(i-\Omega)/2} R_B^{\Omega-i} \sum_{i=1}^i R_L^{i-\Omega}} \right)^{1/m} - h_{co_i} \right]$$

180 where  $h_{co_i}$  is the inflow depth of the  $i$ th-order channel due to water transported from u  
181 reaches, is given as:

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$$h_{co_i} = \left( \frac{q_L n_c A (R_B^{\Omega-i} R_A^{i-\Omega} - P_{OA_i}) \sum_{i=1}^{\Omega} R_L^{i-\Omega}}{S_{c_{\Omega}}^{1/2} B_{\Omega} R_S^{(i-\Omega)/2} R_B^{\Omega-i} \sum_{i=1}^i R_L^{i-\Omega}} \right)^{1/m} \tag{6}$$

Where  $n_c$  is the Manning coefficient of stream,  $B_{\Omega}$  is the width of the stream. The value equal to zero for  $i=1$ .

### 3. Geomorphologic parameters (GP) Horton-Strahler stream-order-law ratios

As observed in the Eqs. (5) and (6), the stream-order-law ratios particularly,  $R_S$ ,  $R_A$ ,  $R_L$ ,  $R_B$  have high importance. These affect the travel time, IUH, and DRH; also, they are computed according to the GP. For this purpose, the stream network is delineated by means of GIS. In the streams are ordered via Horton-Strahler method, and the number, length, and slope of streams are calculated at each order.

In order to calculate the coefficients  $R_S$ ,  $R_A$ ,  $R_L$ ,  $R_B$  coefficients the provided equations according to Table (1) are used.

Table1: The Equations related to Horton-Strahler stream-order-law ratios

Equation Number	Equation	Description
(7)	$R_B = N_{i-1} / N_i$	$N_i$ : Number of $i$ th-order channels
(8)	$R_L = \bar{L}_{c_i} / \bar{L}_{c_{i-1}}$	$\bar{L}_{c_i}$ is the mean Length of $i$ th-order channels
(9)	$R_A = \bar{A}_i / \bar{A}_{i-1}$	$\bar{A}_i$ is the mean area of catchment of order $i$
(10)	$R_S = \bar{S}_{c_i} / \bar{S}_{c_{i-1}}$	$\bar{S}_{c_i}$ : Mean slope of the $i$ th-order streams



197 The value of  $R_B$  is given by the following equation regarding the number of stream seg;  
198 each order:

$$199 \quad R_B = N_{i-1} / N_i, i = 2, 3, \dots, \Omega$$

200  $N_i$  denotes the number of  $i$ th order channels. The length ratio ( $R_L$ ) is:

$$201 \quad R_L = \bar{L}_{c_i} / \bar{L}_{c_{i-1}}, i = 2, 3, \dots, \Omega$$

$\bar{L}_{c_i}$  is the mean length of  $i$ th order channels. Eq. (9) yields the value of  $R_A$ :

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$$203 \quad R_A = \bar{A}_i / \bar{A}_{i-1}$$

204 where  $\bar{A}_i$  is the mean area of catchment of order  $i$ . It should be noted that the mean a  
205 given stream segment is, in fact, a cumulative value, for example, the area of a thi  
206 catchment is a sum of the areas of the first, second and third order streams. Computation  
207 not so easy a task for the GIS users.

208 The value of  $R_S$  depends on the streams slope and is obtained by Eq. (10):

$$209 \quad R_S = \bar{S}_{c_i} / \bar{S}_{c_{i-1}}, i = 2, 3, \dots, \Omega$$

210 where  $\bar{S}_{c_i}$  is the mean slope of the  $i$ th order streams.

211 As a result of experiments in the natural catchments, the following ranges are ol  
212  $3 \leq R_B \leq 5$  and  $1.5 \leq R_L \leq 3.5$ . Slope of the streams and overland planes for different cat  
213 at each order are different. The mean values of these slopes at each order take a cons  
214 time to compute by GIS, especially in large catchments.

215

216 In this research, a new slope ratio named the overland slope ratio ( $R_{SO}$ ) is introduced that  
217 in terms of the mean slope of the overland plane by:

$$218 \quad R_{SO} = \bar{S}_{o_{i-1}} / \bar{S}_{o_i} \quad (11)$$

219 where  $\bar{S}_{o_i}$  is the mean slope of the  $i$ th-order overland plane.

220 In this research we intend to find the relationship between  $R_{SO}$  and the other stream-o-  
221 ratios.

222 ~~Herein, a way for computing GP via regression equations is sought. These equations att~~  
223 ~~regression methods work through statistical analysis of the information of catchments po~~  
224 ~~geomorphologic attributes. The way these equations perform computations will be expl~~  
225 ~~the next sections.~~

#### 226 **4. Case Study**

227 To study the relationship between ~~geomorphologic parameters~~stream-order-law ratios/  
228 knowledge of the GIS based GP-SOLR (i.e the GP-SOLR derived from GIS) of some  
229 catchments is required. This research uses information received from twelve catchr  
230 different countries. Table (1) shows the GIS based GP-SOLR along with stream order :  
231 the case study catchments. The catchments Long chi (~~Shuyou et al. 2010~~); Long men (~~SI~~  
232 ~~al. 2010~~); Chaukhutia (~~Kumar 2014~~); Al-Malaqi (~~Shadeed et al. 2007~~); Debarwa (~~Alemr~~  
233 ~~Mathur 2014~~); Gherghera (~~Alemngus and Mathur 2014~~); San-Hsia (~~Chang and Lee 20~~  
234 ~~Badan (Shadeed et al. 2007)~~; Al-Faria (~~Shadeed et al. 2007~~) were used for train  
235 estimation of regression equations, and the Gagas (~~Kumar and Kumar 2008~~), Heng-Chi (  
236 ~~Chang 2005~~) and Kasilian (~~Sabzevari et al 2013~~) catchments were used for verificatio  
237 suggested equations.

238 The columns Table (~~42~~) (from left to right) illustrate, respectively, the catchment n:  
239 reference, stream order ( $i$ ), number of streams, mean stream length, mean stream are  
240 stream slope, mean overland slope,  $R_B$ ,  $R_L$ ,  $R_A$ ,  $R_S$ , and  $R_{SO}$ .

241 The Heng-Chi catchment is located in northern Taiwan and has an area of 53 km<sup>2</sup> (~~Le~~  
242 The Gagas catchment lies in the middle and outer range of the Himalayas in Uttarakhand  
243 India and has an area of 506 km<sup>2</sup> (~~Kumar and Kumar 2008~~). The Kasilian Catchment is  
244 between 53° 18' E and 53° 30' E longitudes and 35° 58' N to 36° 7' N latitudes in the nortl  
245 and has an area of 67.8 km<sup>2</sup>. Figure (1) shows the Gagas ~~and~~, Kasilian and H  
246 catchments.

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Table 4-2. GP-Geomorphologic characteristics of twelve case study catchments

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Catchment Name	Geomorphologic parameters										
	Order	$N_i$	$\bar{L}_i$	$\bar{A}_i$	$\bar{S}_c$	$\bar{S}_o$	$R_B$	$R_L$	$R_A$	$R_S$	$R_{SO}$
1. Gagas	1	121	1.74	3.02	0.172	0.810	4.8	2.4	5.4	0.4	2.6
	(Kumar and Kumar 2008)	2	23	3.04	18.58	0.141	0.655				
		3	6	7.63	79.22	0.041	0.172				
		4	1	23.4	506	0.017	0.065				
2. Heng-Chi	1	30	0.66	1.043	0.087	0.450	3.3	2.6	4	0.6	1.1
	(Lee and Chang 2005)	2	6	2.74	6.919	0.050	0.419				
		3	2	1.6	19.9	0.012	0.349				
		4	1	4.97	53.23	0.012	0.347				
3. Kasilian	1	42	1.6	0.915	0.241	0.345	3.5	1.5	4.3	0.4	1.1
	(Sabzevari et al 2013)	2	11	1.79	4.813	0.070	0.297				
		3	3	2.45	20.75	0.047	0.263				
		4	1	4.65	67.8	0.008	0.261				
4. San-Hsia	1	69	0.92	1.15	0.161	0.314	4.2	2.9	5	0.4	1.1
	(Chang and Lee 2008)	2	16	2.08	4.99	0.092	0.203				
		3	3	3.88	18.15	0.037	0.364				
		4	1	17.8	125.9	0.013	0.293				
5. Al-Badan	1	41	1.38	1.37	0.170	0.140	4	1.5	4.5	1	1.7
	(Shadeed et al. 2007)	2	6	3.2	10.12	0.092	0.062				
		3	2	5.03	40.73	0.140	0.051				
		4	1	3.17	85	0.135	0.029				
6. Al-Faria	1	49	1.03	0.937	0.154	0.117	4	1.5	4.3	1.1	1.6
	(Shadeed et al. 2007)	2	8	2.12	6	0.085	0.058				
		3	3	3.5	19.4	0.161	0.033				
		4	1	2.62	64	0.125	0.031				
7. Al-Malaqi	1	62	1.92	1.81	0.146	0.140	9	1.3	17	0.8	4.3
	(Shadeed et al. 2007)	2	16	2.61	5.83	0.122	0.063				
		3	1	3.21	185	0.081	0.010				
8. Debarwa	1	23	2.26	5.6	0.032	0.135	4.9	3	6	0.6	1.2
		2	6	4.2	27.8	0.018	0.091				

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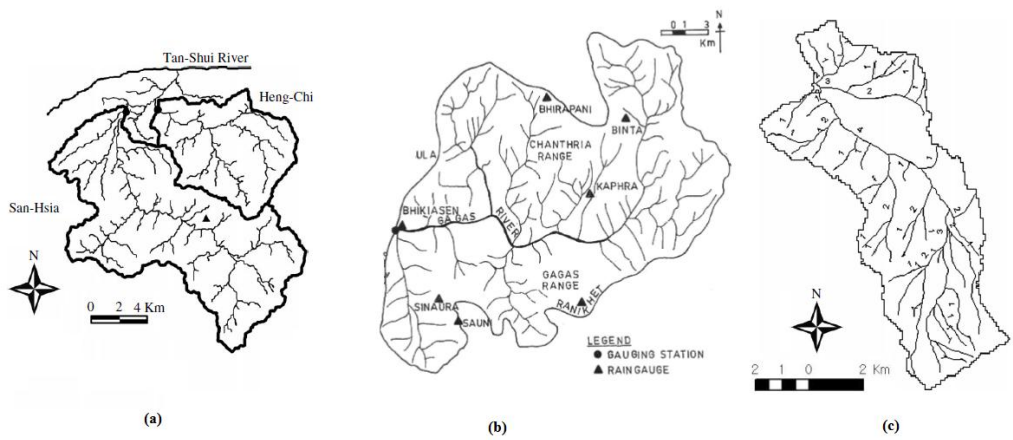
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(Alemngus and Mathur 2014)	3	1	17.7	195	0.010	0.098						
9. Gherghera	1	58	2.45	5.9	0.027	0.136	2.9	1.4	3.3	0.9	1.4	
	2	14	4.19	30.6	0.018	0.087						
(Alemngus and Mathur 2014)	3	5	10.2	101.0	0.010	0.064						
	4	2	4.47	259.9	0.016	0.025						
	5	1	4.19	525.7	0.011	0.117						
10. Long chi	1	46	1.13	2.5	0.210	0.444	3.7	2.4	4	0.6	1.1	
	2	10	3.45	11.8	0.124	0.487						
(Shuyou et al. 2010)	3	4	3.19	32	0.073	0.514						
	4	1	9.94	141.8	0.054	0.364						
11. Long men	1	58	1.31	2.74	0.560	0.256	4	2.2	4.7	0.9	1.8	
	2	13	2.48	12.3	0.560	0.123						
(Shuyou et al. 2010)	3	3	9.33	77.11	0.560	0.056						
	4	1	8.18	246.8	0.385	0.056						
12. Chaukhutia	1	134	1.41	2.27	0.191	0.910	5.3	2.5	5.7	0.5	2.4	
	2	31	2.65	12.28	0.123	0.567						
(Kumar, 2014)	3	7	7.21	60.18	0.041	0.174						
	4	1	20.7	452.3	0.019	0.074						

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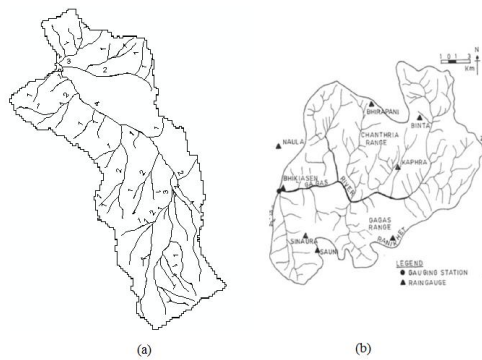


Fig. 1: Drainage network map

a) Heng-Chi catchment b) Gagás catchment ac) Kasilian catchment stream network b) Gagás catchment

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## 54. Relationships of geomorphologic parameters Prediction of Horton-Strahler order-law ratios

### 5.1. Estimation of bifurcation ratio ( $R_b$ )

To estimate the bifurcation ratio of a catchment, the information concerning ~~80-37 wa~~ catchments with areas between 1 km<sup>2</sup> and 600 km<sup>2</sup> were used which had known values of area, with the presumption that  $R_b$  is a function of two variables, catchment area ( $A$ ) main stream length ( $L$ ). With the help of statistical package for the social sciences (S

software (Norusis, 1999, Mohamoud and Parmar, 2006) and using the information c  
catchments an optimum relation was obtained as:

$$R_B = 0.0027A + 3.47 \quad (12)$$

Admittedly, the value of  $R_B$  was not dependent on  $L$ . The correlation coefficient of tl  
equation is 0.8 and the real mean bifurcation ratio of the catchments is 4. -Fig.2 shows t  
linear regression.

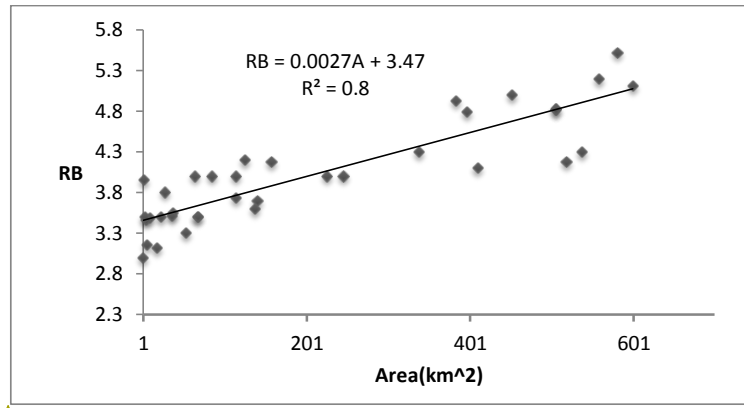


Fig.2: Linear regression between bifurcation ratio and the area of the catchment

Eq. (12) indicates that in small catchments with area less than 600km<sup>2</sup>, the value of  
between 3.47 and 4. It is suggested that Eq. (12) be applied to catchments of areas  
600km<sup>2</sup>. It should be noted that, regarding Eq. (7) and  $R_B$  and  $R_B$ , the values c  $N_i$   
calculated for  $i \leq \Omega$ .  $\Omega$  is the maximum order of the catchment.  $N_{i=\Omega} = 1$  is consid  
 $N_{i-1} = R_B N_i, i \leq \Omega$  (Horton, 1945).

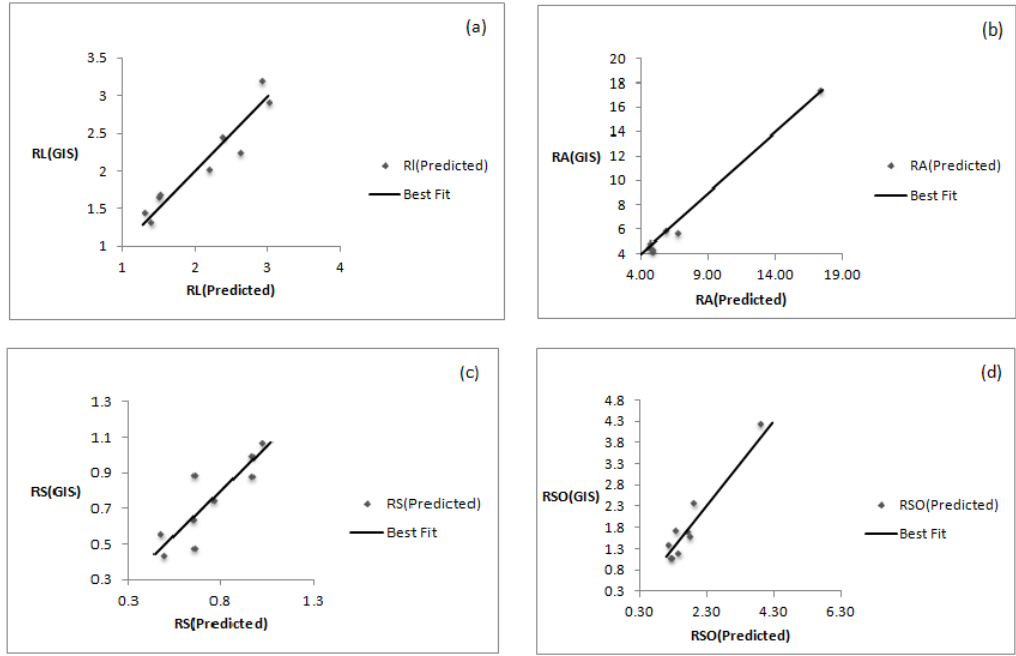
## 5.2. Computation of stream-length Ratio ( $R_L$ )

To calculate the length ratio  $R_L$ , it was taken as a function of the main stream length  
whole catchment area. The fitted regression equation for the nine selected catchments ac  
to Table (1) is, as follows:

$$R_L = 2.59L^{0.41} A^{-0.2} \quad (13)$$

The correlation coefficient is equal to 0.91.

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**Fig3: GIS-based stream-order-law ratios versus predicted SOLR**

**Fig.3a shows the calculated  $R_L$  values based on the Eq.13 in comparison with its real value**

Based on Eq. (8) and  $R_L$ , the values of  $\overline{L}_{c_i}$  are calculated for  $i \leq \Omega$ .  $\overline{L}_{c_\Omega} = L$  is considered

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$$\overline{L}_{c_{i-1}} = \overline{L}_{c_i} / R_L, i \leq \Omega \text{ (Horton, 1945)}.$$

### 5.3. Computation of area ratio ( $R_A$ )

The area ratio was assumed to be a function of the bifurcation ratio and the length ratio fitted equation:

$$R_A = 0.597R_B^{1.553}R_L^{-0.177} \quad (14)$$

The correlation coefficient is 0.99.

Fig.3b shows the calculated  $R_A$  values based on Eq.14 in comparison with its real value.

$\overline{A_\Omega} = A$  is considered and  $\overline{A_{i-1}} = \overline{A_i} / R_A$ ,  $i \leq \Omega$  (Schumm, 1956).

#### 5.4. Computation of stream slope ratio ( $R_S$ )

Stream slope ratio was assumed to be a function of  $R_B$ ,  $R_L$ , and  $R_A$ . Equation (15), correlation coefficient 0.79, represents the fitted regression relation for the data.

$$R_S = 1.198R_B^{1.26}R_L^{-0.97}R_A^{-1.04} \quad (15)$$

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#### 5.5. Computation of overland slope ratio ( $R_{SO}$ )

A nonlinear regression equation consisting of the parameters  $R_B$ ,  $R_L$ ,  $R_A$ , and  $R_S$  was calculate the slope ratio of the overland plane with the fitted relation:

$$R_{SO} = 0.366R_B^2R_L^{-0.58}R_A^{-0.66} \quad (16)$$

The correlation coefficient of Eq. (16) is 0.93, and there is no strong correlation between  $R_S$ .

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Figs.3c and 3d show the fitness amounts of  $R_S$  and  $R_{SO}$  calculated by Eqs. 15 and 16 in comparison with the real values.

By the Eqs. (16) and (11) the slope of overland planes of the catchment could be obtained. It can be noted that the Eqs. (12) to (16) which are gained via the information about nine catchments may be calibrated by adding more data. Given that the length of the main river and the area of the catchments are known, the  $R_B$ ,  $R_L$ ,  $R_A$ ,  $R_S$ , and  $R_{SO}$  ratios can be calculated by Eqs. (12) to

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#### 5. Prediction of catchment's geomorphological information



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320 The area of the catchment, the length and slope of the main river could be determined from  
 21 ~~simple~~ topographic maps of the catchment (Scale: 1:25000 to 1:50000). If a catchment  
 22 maximum stream order  $\Omega$ , it is inferred that the stream should be located at the end of  
 23 catchment with the mean slope ( $\overline{S_{c_\Omega}}$ ) and the mean slope of the lateral overland plane ( $\overline{S_{o_\Omega}}$ )  
 24 For instance, Fig. 2-4 shows a small catchment with three sub-catchment (I, II, I  
 25 maximum stream order is two ( $\Omega=2$ ). The sub-catchment III is created with two lateral c  
 26 planes and stream III is positioned at the end of the main catchment. Fig. 2-4 shows th  
 27 slope of the stream III ( $\overline{S_{c_2}}$ ) and mean slope of the two lateral overland planes ( $\overline{S_{o_2}}$ ).

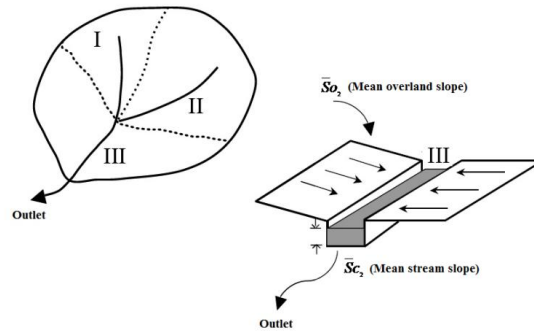


Fig. 24. Catchment with maximum stream order two

330 If the values of the  $\overline{S_{c_\Omega}}$ ,  $\overline{S_{o_\Omega}}$ ,  $R_S$  and  $R_{SO}$  are known, with regard to Eqs. (10) and (11),  
 331 value of the  $\overline{S_{c_i}}$  and  $\overline{S_{o_i}}$  are computable for lower orders  $i < \Omega$  ( $\overline{S_{c_{i-1}}} = \overline{S_{c_i}} / R_S$ ,  $\overline{S_{o_{i-1}}} = \overline{S_{o_i}} R_{SO}$ )

332 To calculate the value of  $P_{x_i x_j}$  in Eq. (1) the following equation is used:

$$333 \quad \underline{\underline{P_{x_i x_j} = N_{i,j} / N_i}} \quad (1)$$

334 where  $N_{i,j}$  is number of  $i$ th order stream contributing the flow to  $j$ th order stream;  $N_i$   
 335 number of  $i$ th order channel. The value of  $N_i$  is computable by the bifurcation ratio, but t  
 336 the parameter  $N_{i,j}$  the following equation is suggested:

337

$$N_{i,j} = 2N_i \exp(-0.64j)$$

which is obtained through nonlinear regression of the stream network data by geomorphologic parameters of the Kasilian and the Gagas catchments. In the cat possessing DEM one needs to delineate stream network and order them by GIS s however, calculation of  $N_{i,j}$  should be done manually and rendered by GIS operator w time-consuming and difficult task.

#### 66. Effect of ratios $R_B$ , $R_L$ , $R_A$ , $R_S$ and $R_{SO}$ on DRH

In the previous section of this study, empirical equations were presented to geomorphologic ratios. Now, we apply the GIUH model to look into sensitivity analysis ratios and their effects on DRH and on peak flood. To this end, the information of the catchment was utilized.

Fig. 35a illustrates the effect of bifurcation ratio upon DRH of the Kasilian catchment May, 1993.

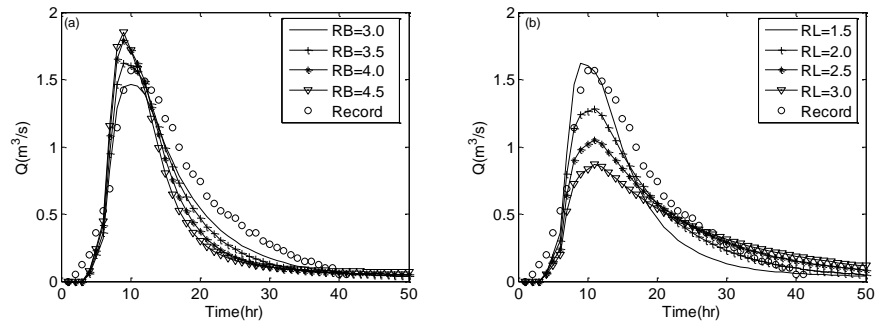


Fig. 35. Effect of  $R_B$  and  $R_L$  on direct runoff hydrograph DRH of the (Kasilian catchment)

The values of bifurcation coefficient 3, 3.5, 4, and 4.5 with 0.5 units increment were computed for the Kasilian catchment, and the number of streams and the values of input parameters of the GIUH model were computed and inserted to the model. The effect of  $R_B$  on shape of hydrograph

and peak of the runoff is seen in Fig. (3a5a). The results of the model are compared with recorded runoff hydrographs.

To determine the effect of different values of  $R_B$  on the peak of runoff, the following equation relative sensitivity was used:

$$S_r = \frac{O_2 - O_1}{P_2 - P_1} (\bar{P} / \bar{O})$$

(4719)

where  $O$  and  $P$  represent particular model outputs and parameters, respectively. So,  $S_r$  is percentage change in  $O$  for a 1% change in  $P$ .  $\bar{P}$  is given by  $(P_1 + P_2)/2$  and  $\bar{O}$  is  $(O_1 + O_2)/2$ . Results confirmed that the least computational error in peak discharge relative to observed peak discharge was shown by  $R_B = 3.5$  with 3.5%. The actual  $R_B$  for the catchment is also 3.5. The mean relative sensitivity of  $R_B$  derived from Eq. (4719) is 0.56. Fig. (3b5b) shows the effect of  $R_L$  on DRH of the Kasilian catchment. The values of  $R_L$  were taken as 1, 1.5, 2, and 2.5 with a 0.5 increment. According to the results,  $R_L = 1.5$  h shows the least error in peak discharge with 3.6% value. The actual  $R_L$  of the catchment is 1.46, mean relative sensitivity of  $R_L$  amounts to 0.92. The larger the value of  $R_L$ , the higher the peak runoff is affected more by length ratio relative to bifurcation ratio, a fact seen also in (35). The next section of the paper was dedicated to the effects of area ratio on the runoff. The values of area ratio were regarded to be between 3 and 6 with 1 unit in values. Figure (46) depicts the effect of area ratio on DRH.

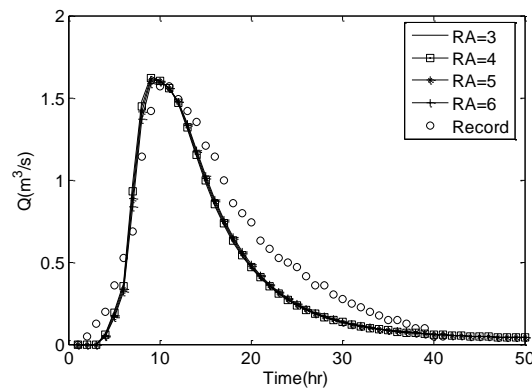


Fig.46. Effect of area ratio on direct runoff hydrographDRH of the (Kasilian catchment)

As indicated by the results, the area ratio has had a slight effect on the runoff peak, alterations of this ratio do not noticeably influence the shape of hydrograph and flood pea Fig. (5a7a) shows how  $R_S$  affects DRH for the values 0.1, 0.4, 0.7, and 1 with a 0.3 incr

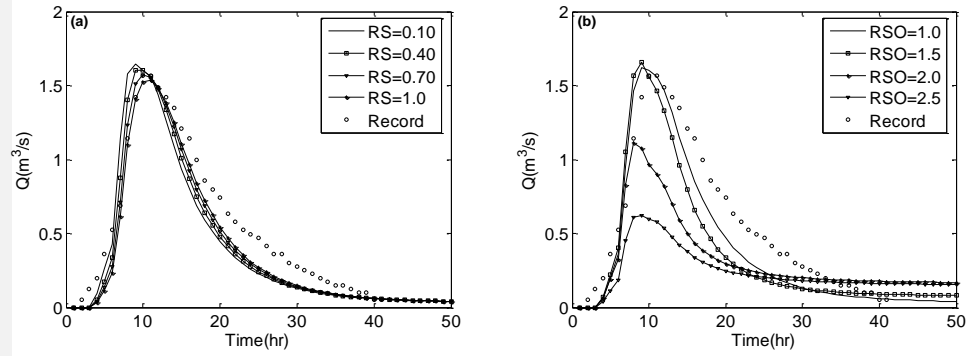


Fig. 57. Effect of  $R_S$  and  $R_{SO}$  on DRH direct runoff hydrograph of the (Kasilian catchment)

The least error is 0.47 which corresponds to the ratio (0.7) while the actual slope ratio Kasilian catchment is 0.38. Also, the mean relative sensitivity ratio is 0.042. The results that this parameter has little effect on runoff peak, too.

Figure (5b7b) shows the influence of  $R_{SO}$  on DRH for values of 1, 1.5, 2, and 2.5 with in as 0.5. The least error relates to the ratio 1 which is 3.54%, whilst that of Kasilian ca would be 1.1, and the mean relative sensitivity ratio 1.33. According to the results, the p:  $R_{SO}$  has remarkable effect on runoff peak.

According to the overall results, the relative sensitivity ratio of  $R_B$ ,  $R_L$ ,  $R_A$ ,  $R_S$ , and  $R_{SO}$  0.92, 0.01, 0.042, and 1.33 respectively. The most effect concerns, correspondingly overland slope ratio, length ratio, bifurcation ratio, slope ratio, and area ratio.

~~To calculate the value of  $P_{x_i x_j}$  in Eq. (1) the following equation is used:~~

$$P_{x_i x_j} = N_{i,j} / N_i$$

396 ~~where  $N_{i,j}$  is number of  $i$ th order stream contributing the flow to  $j$ th order stream;  $i$~~   
397 ~~number of  $i$ th order channel. The value of  $N_i$  is computable by the bifurcation ratio, but t~~  
398 ~~the parameter  $N_{i,j}$ , the following equation is suggested:~~

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$$N_{i,j} = 2N_i \exp(-0.64j)$$

402 ~~which is obtained through nonlinear regression of the stream network data by~~  
403 ~~geomorphologic parameters of the Kasilian and the Gagas catchments. In the cat~~  
404 ~~possessing DEM one needs to delineate stream network and order them by GIS s~~  
405 ~~however, calculation of  $N_{i,j}$  should be done manually and rendered by GIS operator w~~  
406 ~~time consuming and difficult task.~~

## 407 **7.7. Verification**

### 08 **7.1. Validation of stream-order-law ratios**

409 In the previous sections, equations were proffered for computation of stream-order-la  
410 based on GP in nine different catchments in the world. For verification of the result  
411 regression equations the GP of three catchments Gagas, Heng-Chi, and Kasilian were app  
  
412 Table (23) lists the GP as well as stream-order-law ratios of the three selected catchmer  
413 Eqs. (12) to (16). The table (23) also provides the ~~observed~~-values of stream order ra  
414 their computational errors.

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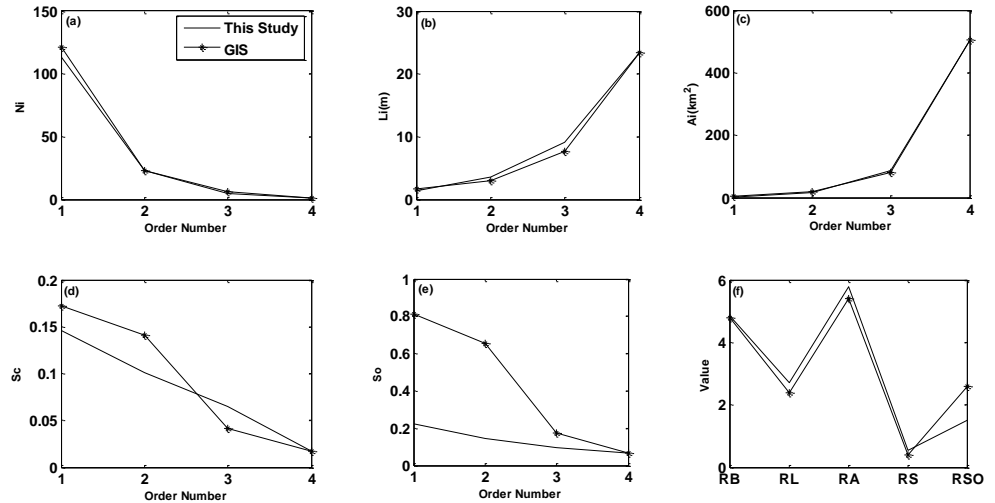


Fig. 68. Verification of geomorphological parameters GP in Gagas catchment

In order to calculate the error related to the stream- order-law ratios the Eq.20 below has used:

$$Error\% = 100 * (R_p - R_{GIS}) / R_{GIS} \quad (20)$$

Where  $R_p$  is predicted stream-order ratio and  $R_{GIS}$  is GIS-based stream-order ratio.

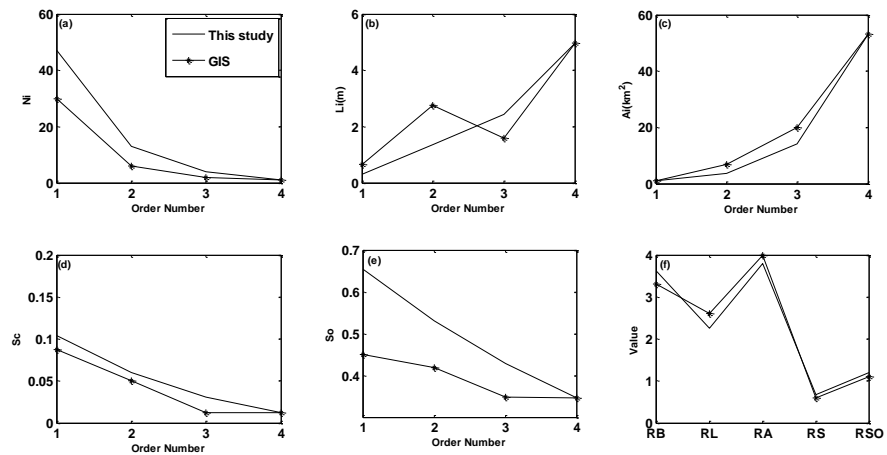
Table 2-3 Calculated geomorphological parameters GP of the Gagas, Heng-Chi, and Kasilian catchm

Catchment Name	<u>Predicted</u> Geomorphologic parameters										
	Order	$N_i$	$\bar{L}_i$	$\bar{A}_i$	$\bar{S}_c$	$\bar{S}_o$	$R_B$	$R_L$	$R_A$	$R_S$	$R_{SO}$
1. Gagas	1	113	1.38	2.6	0.146	0.222	4.84	2.72	5.78	0.53	1.5
	2	23	3.54	15.1	0.101	0.147					
	3	5	9.10	87.5	0.065	0.098					
	4	1	23.40	506.0	0.017	0.065					
GIS Results							4.80	2.40	5.40	0.40	2.60
%Error							0.40	13.7	7.6	21.0	41.4
2. Heng-Chi	1	47	0.32	1.0	0.104	0.654	3.61	2.26	3.80	0.68	1.2
	2	13	1.34	3.7	0.060	0.530					

	3	4	2.43	14.0	0.031	0.429					
	4	1	4.97	53.2	0.012	0.347					
GIS Results							3.30	2.60	4	0.60	1.10
%Error							9.4	13.7	5.0	13.3	9.1
3. Kasilian	1	49	0.49	1.1	0.109	0.563	3.65	2.09	3.92	0.72	1.3
	2	13	1.03	4.4	0.073	0.436					
	3	4	2.19	17.3	0.038	0.337					
	4	1	4.65	67.8	0.008	0.261					
GIS Results							3.5	1.5	4.3	0.4	1.1
%Error							4.3	43.2	8.8	89.5	18.2

423

424 Figs [68](#), [to 8-10](#) depict the GIS based and computational GP concerning the three ca  
425 catchments.



426

427 Fig. [79](#). Verification of [geomorphological parameters GP](#) in Heng-Chi catchment

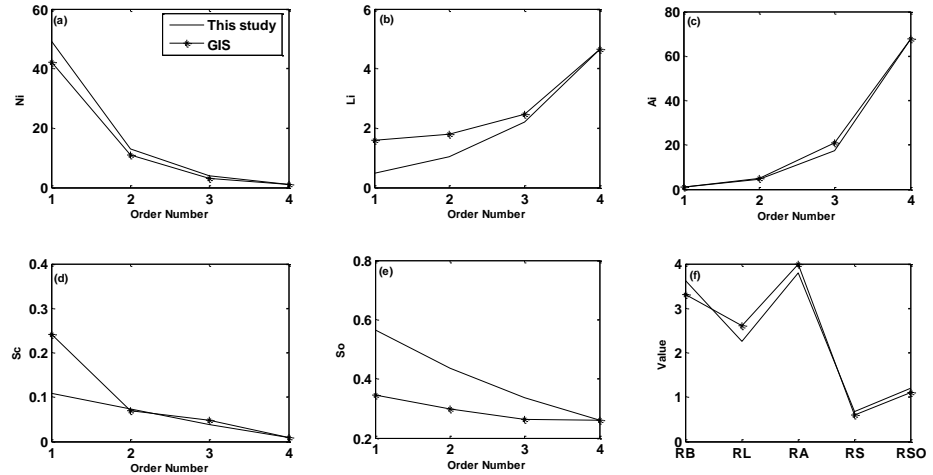


Fig.8.10. Verification of ~~GP-geomorphological parameters in~~ Kasilain catchment

The mean errors of regression equations in estimation of  $R_B$ ,  $R_L$ ,  $R_A$ ,  $R_S$ , and  $R_{SO}$  in t selected catchments are, respectively, 4.7%, 23.5%, 7.1%, 41.3%, and 22.9%.

The greatest errors of the model emerged in estimation of, respectively,  $R_S$ ,  $R_L$ ,  $R_{SO}$ ,  $R_A$ , As observed in Fig. (5a7a), the stream slope ratio has a slight affect on runoff, so its err be ignored. Regarding high sensitivity of the length and overland slope ratios their erro from 23 to 24 percent and it is recommended that the joint effects of all the ratios on DR selected catchments be considered.

## 7.2. Validation of catchment's direct runoff

In the previous sections, the influences of ~~GPSOLR~~ on runoff were pondered separately, GP of the three catchments were estimated via the regression equations. To study accurac estimations more deeply it is better to estimate the DRH using GIUH model. For this taking the information about excess rainfall hyetograph and recorded runoff of the Kasi the Heng-Chi catchments into consideration, we turn to verification of the predicted ~~direx~~ for the two catchments.

The model GIUH was employed in two cases, one in which ~~geomorphologic paramete~~ are GIS based and the other where empirical regression equations (~~GIS-unsupportedthi~~



are concerned for the Kasilian and the Heng-Chi catchments. The results of the model case were compared with those of observed runoff recorded. Since the observed rainfall data of Gagas catchment were not available, this catchment was dispensed in ver phase. Figure 9-11 shows the results of GIUH model for DRH estimation in Kasilian ca for two events on 10<sup>th</sup> May 1992 and 4<sup>th</sup> May1993. Also,-Fig. (4012) illustrates those i Chi catchment for two events July 1996 and October 2000.

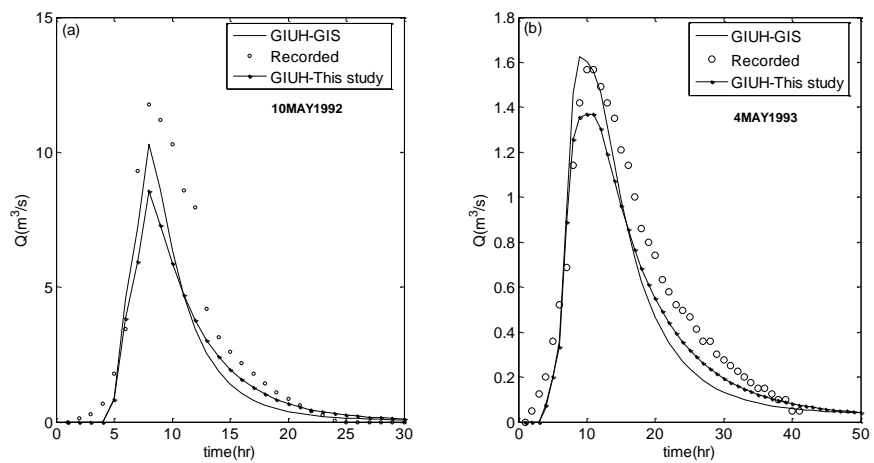


Fig.9.11. Estimation of Kasilian direct runoff hydrographDRH by GIUH model

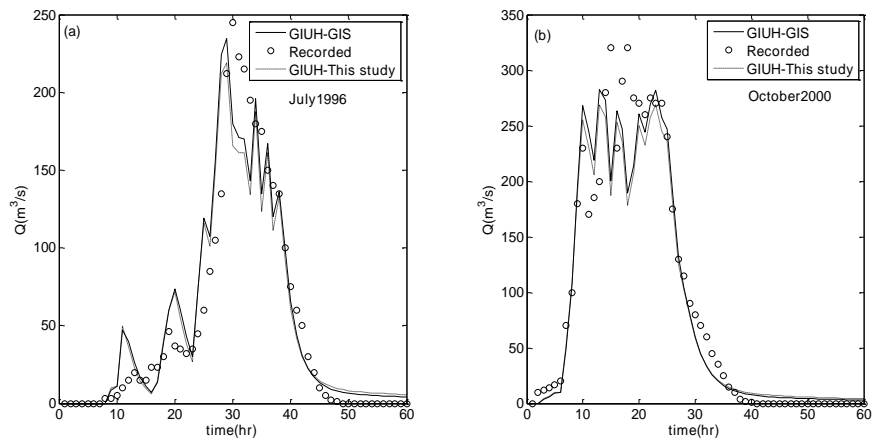


Fig.40.12. Estimation of Heng-Chi DRH-direct runoff hydrograph by GIUH model

456 To validate the fitness of the model for the Kasilian and Heng-Chi catchments, three c  
 457 statistical measures were ~~used-used~~ the coefficient of efficiency (*CE*), ~~Root-root~~ mean  
 458 error (*RMSE*), and ~~Relative-relative~~ error in peak (*REP*).

459 Estimation of these three parameters ~~is-are~~ carried out by the following equations:

460 
$$CE = 1 - \frac{\sum_{t=1}^n [Q_r - Q_s]^2}{\sum_{t=1}^n [Q_r - \bar{Q}_r]^2}$$

461 
$$RMSE = \left[ \frac{1}{n} \sum_{i=1}^n (Q_r - Q_s)^2 \right]^{0.5}$$

462

463 
$$REP = 100 \times [Q_{p_s} - Q_{p_r}] / Q_{p_r}$$
 (

464 where  $Q_r$  is the recorded discharge at time  $t$ ;  $Q_s$  is the simulated discharge at time  $t$ ;  $\bar{Q}_r$   
 465 mean recorded discharge during the storm event;  $n$  is the number of discharge records du  
 466 storm event;  $Q_{p_s}$  is the peak discharge of the simulated hydrograph and  $Q_{p_r}$  is the 1  
 467 peak discharge.

468 Table (34) gives the values of *REP*, *CE*, and *RMSE* calculated for the two selected catch  
 469 GIS-supported and ~~GIS-unsupported-proposed method~~ (this study) cases.

Table 34. Validation result of the GIUH model

July 1996	<i>REP</i> %	<i>CE</i>	<i>RMSE</i>
GIS	4.18	0.87	24.54
This study	10.62	0.86	25.44
October 2000			
GIS	11.81	0.93	31.22
This study	15.99	0.92	32.25
10 May 1992			
GIS	12.68	0.81	1.13
This study	27.33	0.76	1.26
4 May 1993			
GIS	3.5	0.87	0.10
This study	12.6	0.91	0.10

472 It is concluded that the computational error values of runoff peak ( $REP\%$ ) that could be  
473 (in this study) for the four rainfall-runoff events are, on average, 10% more than t  
474 resulting from actual information (GIS support). As seen in Figs (911) and (4012), the r  
475 the GIUH model in the two cases concerning GIS and empirical equations are very close  
476 other.  $CE$  and  $RMSE$  are near-valued as well. The mean  $CE$  of the model was computed  
477 four events as 0.87 which is a satisfactory value.

## 478 8. Summary and conclusion

479 In this research, experimental equations were presented to work out geomorphologic and  
480 order-law ratios parameters of watersheds of less than 600 km<sup>2</sup> area. These equations are  
481 in accordance with the nonlinear regression method fitted to the stream-order-law  
482 geomorphologic parameters of nine different catchments of the world. The equations we  
483 under verification in three other selected catchments, and their results were compared wi  
484 calculated from GIS.

485 The geomorphologic parameters and stream-order-law ratios of three catchments Gagas  
486 Chi, and Kasilian were determined based on the experimental equations given in this r  
487 and compared with their actual results. The average errors of the model in estimation of  
488  $R_A$ ,  $R_S$ , and  $R_{SO}$  in the three case study catchments were 4.7%, 23.5%, 7.1%, 41.3%, and  
489 respectively.

490 ~~Finally, direct runoff hydrograph was estimated by GIUH with regard to the geomor~~  
491 ~~data computed for the three catchments, and then compared to the observed values. Sensi~~  
492 bifurcation ratio ( $R_B$ ), length ratio ( $R_L$ ), area ratio ( $R_A$ ), stream slope ratio ( $R_S$ ), and c  
493 slope ratio ( $R_{SO}$ ) to runoff of Kasilian catchment were investigated. It is shown that the  
494 sensitivity of  $R_B$ ,  $R_L$ ,  $R_A$ ,  $R_S$ , and  $R_{SO}$  was 0.56, 0.01, 0.92, 0.042, and 1.33, respecti  
495 greatest effect was related to, respectively, the overland slope ratio, length ratio, and bif  
496 ratio, and the least effect was related to area ratio, and streams slope ratio.

497 ~~The geomorphologic parameters of three catchments Gagas, Heng Chi, and Kasili~~  
498 ~~determined based on the experimental equations given in this research, and compared w~~

499 ~~actual results. The average errors of the model in estimation of  $R_B$ ,  $R_L$ ,  $R_{A1}$ ,  $R_{A2}$ , and  $R_{A3}$~~   
500 ~~three case study catchments were 4.7%, 23.5%, 7.1%, 41.3%, and 22.9%, respectively.~~

501 Finally, direct runoff hydrograph was estimated by GIUH with regard to the geomor  
502 data computed for the three catchments, and then compared to the observed values.

503 Lastly, the estimated ~~stream-order-law ratios geomorphologic parameters was were~~ input  
504 GIUH model and the values of direct runoff hydrograph of two catchments Kasilian an  
505 Chi were calculated and compared with those of observed direct runoff. According to the  
506 the computational averaged error values of runoff peak ( $REP\%$ ) for the four rainfa  
507 events are, on average, 10% more than the error resulting from actual informatio  
508 Supported). The results of the GIUH model in the two cases concerning GIS and without  
509 very close to each other.  $CE$  and  $RMSE$  in the two cases are near-valued as well. Th  
510 coefficient of efficiency of the model was computed for the four events as equal to 0.87.

511

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